ABBREVIATIONS:

AB	ANCHOR BOLT
ABV	ABOVE
ALT	ALTERNATE
ARCH	ARCHITECTURAL
BLKG	BLOCKING
BLW	BELOW
BM	BEAM
BOT	BOTTOM
BRG	BEARING
BSMT	BASEMENT
BTWN	BETWEEN
CJ	CONSTRUCTION JOINT
CL	CENTER LINE
CLR	CLEAR
CNSK	COUNTERSINK
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
CP	COMPLETE PENETRATION
d	PENNY (NAIL SIZE)
DIA	DIAMETER
DWG	DRAWING
DIM	DIMENSION
DO	DITTO
DTL	DETAIL
EA	EACH
EF	EACH FACE
EF/EW	EA FACE, EA WAY
EJ	EXPANSION JOINT
EL	ELEVATION
EMBED	EMBEDMENT
EN	EDGE NAILING
EQ	EQUAL
EQUIP	EQUIPMENT
ES	EACH SIDE
EW	EACH WAY
(E)	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FDN	FOUNDATION
FLR	FLOOR
FRMG	FRAMING
FT	FOOT OR FEET
FTG	FOOTING
GA	GAUGE
GB	GRADE BEAM
GLULAM	GLUE-LAMINATED
GLB	GLULAM BEAM
HD	HOLDOWN
HDG	HOT-DIP GALVANIZED
HDR	HEADER
HF	HARDY FRAME
HGR	HANGER
HORIZ	HORIZONTAL
HSB	HIGH-STRENGTH BOLT
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIPS, KILOPOUNDS
LAM	LAMINATE
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONGIT	LONGITUDINAL
LP	LOW POINT
MAX	MAXIMUM
MB	MACHINE BOLT
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
(N)	NEW
N/A	NOT APPLICABLE
NTS	NOT TO SCALE
O/	OVER
OC	ON CENTER
OH	OPPOSITE HAND
OPNG	OPENING
PL	PLATE
PLCS	PLACES
PLF	POUNDS PER LINEAR FOOT
PLY	PLYWOOD
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PTDF	PRESSURE TREATED DOUGLAS FIR
R	RADIUS
RDWD	REDWOOD
REF	REFERENCE
REINF	REINFORCED/REINFORCING
REQD	REQUIRED
RET	RETAINING
RS	ROUGH SAWN
SAD	SEE ARCHITECTURAL DWGS
SCHED	SCHEDULE
SECT	SECTION
SHT	SHEET
SHTG	SHEATHING
SIM	SIMILAR
SIMP	SIMPSON
SN	SOLE NAILING (FASTENING)
SOG	SLAB-ON-GRADE
SPCG	SPACING
SPEC	SPECIFICATION
STAGG	STAGGER, STAGGERED
STIFF	STIFFENER
STRUCT	STRUCTURAL
SYMM	SYMMETRICAL
SW	SHEARWALL
T&B	TOP AND BOTTOM
T&G	TONGUE AND GROOVE
TBD	TO BE DETERMINED
THK	THICK
THRD	THREADED
THRU	THROUGH
TN	TOE NAIL
TOF	TOP OF FOOTING
TOS	TOP OF SLAB
TRANSV	TRANSVERSE
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
VB	VAPOR BARRIER
VERT	VERTICAL
VIF	VERIFY IN FIELD
W/	WITH
WP	WORK POINT

GENERAL

1. THE CONTRACTOR SHALL COORDINATE ALL STRUCTURAL DOCUMENTS WITH ALL OTHER DISCIPLINES AND REPORT ALL DISCREPANCIES TO THE ENGINEER FOR RESOLUTION. ALL DIMENSIONS ARE TO BE CHECKED AND VERIFIED WITH THE ARCHITECTURAL DRAWINGS.

2. DIMENSIONS REFER TO ROUGH CONCRETE SURFACES, FACE OF STUDS, FACE OF CONCRETE BLOCK, TOP OF SHEATHING, OR TOP OF SLAB UNLESS OTHERWISE NOTED. DO NOT USE SCALED DIMENSIONS. WHERE NO DIMENSION IS PROVIDED OR CAN BE DERIVED, CONSULT WITH ARCHITECT PRIOR TO CONSTRUCTION.

3. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS AND/OR SPECIFICATIONS FOR SIZE AND LOCATION OF PIPE, VENT, DUCT, AND OTHER OPENINGS, AND ANY OTHER DETAILS NOT SHOWN ON THE STRUCTURAL DRAWINGS. SEE ARCHITECTURAL DRAWINGS FOR WATERPROOFING, DAMPPROOFING, AND DRAINAGE REQUIREMENTS. WHERE SUCH INFORMATION IS SHOWN ON THE STRUCTURAL DRAWINGS IT IS FOR INFORMATION ONLY.

4. SEE CIVIL AND/OR SITE DRAWINGS AND DETAILS FOR GRADING, PAVING, DRAINAGE, WATER AND SANITARY SERVICE AND OTHER CIVIL ENGINEERING ITEMS.

5. TYPICAL DETAILS AND NOTES SHOWN ON THESE SHEETS SHALL APPLY UNLESS SPECIFICALLY SHOWN OR NOTED OTHERWISE. CONSTRUCTION DETAILS NOT FULLY SHOWN OR NOTED SHALL BE SIMILAR TO DETAILS FOR SIMILAR CONDITIONS.

6. ALL WORK AND CONSTRUCTION SHALL COMPLY WITH ALL APPLICABLE BUILDING CODES, REGULATIONS, AND SAFETY REQUIREMENTS.

7. THE PROJECT PLANS SHOW THE COMPLETED STRUCTURE. MEANS AND METHODS OF CONSTRUCTION AND THE SAFETY OF PERSONNEL AND PROPERTY DURING CONSTRUCTION SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY. THE CONTRACTOR SHALL DESIGN AND PROVIDE ADEQUATE SHORING, BRACING, AND FORMWORK, ETC. AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION. WHERE NECESSARY, THE CONTRACTOR SHALL RETAIN A CALIFORNIA LICENSED CIVIL ENGINEER TO DESIGN AND DETAIL SHORING, UNDERPINNING, BRACING, FALSEWORK, AND FORMWORK.

8. THE CONTRACTOR SHALL INFORM THE ENGINEER IN WRITING OF ANY DISCREPANCIES OR OMISSIONS NOTED ON THE DRAWINGS OR SPECIFICATIONS OR OF ANY VARIATIONS REQUIRED TO CONFORM WITH CODE, RULES OR REGULATIONS.

9. ANY DEVIATION FROM THE STRUCTURAL PLANS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING APPROVAL FROM THE BUILDING DEPARTMENT FOR ALL STRUCTURAL CHANGES.

10. AFTER ISSUANCE AND PRIOR TO CONSTRUCTION AND ORDERING OF MATERIALS, THE CONTRACTOR SHALL ATTEND A PRE-CONSTRUCTION MEETING WITH THE ARCHITECT AND ENGINEER. THE ARCHITECT AND ENGINEER SHALL BE NOTIFIED OF THE DATE OF THE MEETING TEN WORKING DAYS PRIOR.

DESIGN DATA

DESIGN CODE: 2007 CALIFORNIA BUILDING CODE AS MODIFIED BY LOCAL BUILDING CODES AND PROVISIONS.

LIVE LOADS:

WIND LOADS:

- 3s GUST WIND SPEED: 85 MPH
- EXPOSURE FACTOR: B
- IMPORTANCE FACTOR: 1.0

SEISMIC LOADS:

- SEISMIC DESIGN CATEGORY: E
- IMPORTANCE FACTOR I = 1.0
- SPECTRAL RESPONSE ACCELERATIONS: Ss=1.58g S1=0.817g
- SITE CLASS: C

STRUCTURAL COEFF. R = 6.5

SEISMIC COEFFICIENTS: Sds=1.054 V=0.264W (ULS)

FOUNDATIONS

1. FOUNDATION DESIGN CRITERIA ARE TAKEN FROM THE SOILS REPORT BY JENSEN VAN LIENDEN ASSOCIATES DATED JULY 18, 2008. DESIGN VALUES USED ARE AS FOLLOWS:

   - PASSIVE PRESSURE: = 300 PCF
   - ALLOWABLE BEARING PRESSURE: = 1000 PSF (DL + LL)

   (SEE PROJECT GEOTECH REPORT FOR ADDITIONAL SITE PREPARATION AND OTHER REQUIREMENTS.)

2. FOOTINGS SHALL EXTEND TO SUCH DEPTH AS TO BEAR ON FIRM UNDISTURBED SOIL. ALL ABANDONED FOOTINGS, UTILITIES, ETC. SHALL BE REMOVED. FOOTING DEPTHS SHOWN ON DRAWINGS ARE MINIMUM DEPTHS. UNDER NO CIRCUMSTANCES SHALL EXISTING FOOTINGS BE UNDERMINED. ADJACENT EXCAVATIONS SHALL NOT BE EXTENDED BELOW THE BOTTOM OF EXISTING FOOTINGS OR SLABS UNLESS OTHERWISE SHOWN ON THE DRAWINGS.

CONCRETE

1. ALL CONCRETE SHALL BE REGULAR WEIGHT, HARD-ROCK READY-MIXED CONFORMING TO ASTM C94. ALL STRUCTURAL CONCRETE SHALL USE ASTM C-150 TYPE II LOW ALKALI PORTLAND CEMENT. FLY ASH SHALL MEET ASTM C618, CLASS F DESIGNATION.

2. ALL AGGREGATES SHALL BE NORMAL-WEIGHT AND SHALL CONFORM TO ASTM C-33. SLABS ON GRADE SHALL BE DESIGNED FOR LOW SHRINKAGE. ACCEPTABLE AGGREGATES FOR LOW SHRINKAGE INCLUDE KAISER CLAYTON, GRANITE ROCK, OR LIMESTONE. FINE AGGREGATES FOR LOW SHRINKAGE INCLUDE OLYMPIA OR FELTON SANDS. ALTERNATIVE AGGREGATES MAY BE SUBMITTED FOR APPROVAL PROVIDED THEY PRODUCE A CONCRETE MIX WITH A SHRINKAGE LIMITATION OF 0.04% AFTER 28 DAYS OF DRYING.

3. IF HIGHER SLUMP IS REQUIRED, MODIFY MIX WITH A TYPE A WATER REDUCER. DO NOT ADD WATER TO ACHIEVE A HIGHER SLUMP.

4. USE NC334 ACCELERATOR BY MASTER BUILDER, OR DARASET 200 BY GRACE, OR COMPARABLE.

5. CONCRETE SHALL HAVE THE FOLLOWING CHARACTERISTICS:

   - STRENGTH @ 28 DAYS: 3000PSI
   - MAX. SLUMP: 4"
   - MIN CEMENTITIOUS CONTENT: 6 SACKS w/ 50% FLY ASH REPLACEMENT
   - MAX WATER/CEMENT RATIO: 0.45
   - MAX AGGREGATE SIZE: ¾"

6. ALL CONCRETE SHALL BE CAST-IN-PLACE. NO OTHER METHOD OF PLACEMENT SHALL BE USED WITHOUT THE EXPRESS WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER. IF ANY OTHER METHOD OF PLACEMENT IS DESIRED, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED 14 DAYS PRIOR TO INSTALLATION OF ANY FORMWORK OR REINFORCEMENT. THE CONTRACTOR SHALL MEET ANY ADDITIONAL REQUIREMENTS (E.G. SPECIAL INSPECTION, FORMWORK, & REINFORCEMENT) IMPOSED BY THE STRUCTURAL ENGINEER FOR ANY ALTERNATE PLACEMENT METHODS.

7. FORMS SHALL BE PROPERLY CONSTRUCTED CONFORMING TO THE CONCRETE SURFACE SHOWN ON THE PLANS, SUFFICIENTLY TIGHT TO PREVENT LEAKAGE, SUFFICIENTLY STRONG, AND BRACED TO MAINTAIN THEIR SHAPE AND ALIGNMENT UNTIL NO LONGER NEEDED TO SUPPORT THE CONCRETE. FORMS FOR EXPOSED CONCRETE SHALL PRODUCE A FINISHED SURFACE WHICH IS SMOOTH AND FREE FROM BLEMISHES. MOISTEN FORMS PRIOR TO PLACEMENT OF CONCRETE. FORMS AND SHORING SHALL NOT BE REMOVED UNTIL CONCRETE HAS ATTAINED SUFFICIENT STRENGTH TO WITHSTAND ALL LOADS TO BE IMPOSED WITHOUT EXCESSIVE STRESS, CREEP, OR DEFLECTION. LEAVE FORMS ON FOR MIN 7 DAYS, AND KEEP WET.

8. AFTER REMOVING FORMS, COVER CONC WITH PLASTIC MIN 7 DAYS, AND KEEP FULLY WET THEN APPLY AMPLE CURING COMPOUND.

9. PIPES OTHER THAN ELECTRICAL CONDUIT SHALL NOT BE EMBEDDED IN CONCRETE EXCEPT WHERE SPECIFICALLY APPROVED BY THE ENGINEER. SEE TYPICAL DETAILS FOR DETAILS OF PIPE SLEEVES EMBEDDED IN CONCRETE. ELECTRICAL CONDUIT EMBEDDED IN CONCRETE SHALL NOT BE LARGER THAN 1-1/2".

10. CONSTRUCTION JOINTS IN FOOTINGS SHALL BE LOCATED BY THE CONTRACTOR EXCEPT AS SHOWN SPECIFICALLY ON THE DRAWINGS. UNLESS OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS, ALL JOINTS SHALL BE ROUGHENED AND KEYED AND EXISTING CONCRETE SURFACES THOROUGHLY CLEANED AND ROUGHENED TO RECEIVE NEW CONCRETE. ROUGHENED INTERFACE SHALL HAVE AN AMPLITUDE OF 1/4".

11. REFER TO ARCHITECTURAL PLANS AND DETAILS FOR CONCRETE FINISHES, CHAMFERS, RADII, OR OTHER FINISH FEATURES.

REINFORCEMENT

1. ALL DEFORMED REINFORCEMENT FOR CONCRETE OR MASONRY SHALL CONFORM TO ASTM A-615 EXCEPT AS NOTED BELOW. ALL DEFORMED BARS SHALL BE 60 GRADE EXCEPT #3 AND #4 BARS MAY BE 40 GRADE. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.

2. UNLESS SHOWN ON THE STRUCTURAL PLANS, REINFORCEMENT SHALL NOT BE WELDED. ANY REINFORCEMENT TO BE WELDED MUST CONFORM TO ASTM A-706 GRADE 60. ALL WELDS FOR REINFORCEMENT SHALL BE PERFORMED BY A QUALIFIED WELDER AND PER PROCEDURES DEFINED IN AWS D1.4. ALL REBAR WELDING SHALL BE SUBJECT TO SPECIAL INSPECTION AS DESCRIBED IN CHAPTER 17 OF THE CALIFORNIA BUILDING CODE.

3. ALL REINFORCEMENT SHALL BE SECURELY TIED IN PLACE SO AS TO MAINTAIN POSITION BEFORE, DURING AND AFTER PLACEMENT OF CONCRETE. BARS AND WIRE MESH SHALL BE SUPPORTED ON WELL-CURED BLOCKS OR PLASTIC CHAIRS AND SECURELY TIED IN PLACE WITH #16 ANNEALED IRON WIRE.

4. ALL STRUCTURAL CONCRETE IS TO BE REINFORCED UNLESS SHOWN OTHERWISE. PROVIDE DOWELS TO MATCH ALL REINFORCEMENT AT CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.

5. ALL BARS WITHOUT LAP LENGTHS SHOWN SHALL BE CONTINUOUS. ALL LAP SPLICES SHALL BE CLASS B TENSION SPLICES WITH LENGTHS AS SHOWN ON THE TYPICAL DETAILS, UNLESS OTHERWISE NOTED. ALL BARS ENDING AT THE FACE OF A WALL, COLUMN, OR BEAM SHALL EXTEND TO WITHIN 2" OF THE FAR FACE AND TERMINATE IN A STANDARD HOOK, UNLESS OTHERWISE NOTED.

6. STANDARD HOOKS SHALL BE AS SHOWN IN THE TYPICAL DETAILS. ALL REBAR BENDS SHALL BE MADE COLD AND BARS SHALL NOT BE REBENT OR BENT AFTER A PORTION OF THE BAR IS ENCASED IN CONCRETE UNLESS APPROVED BY THE STRUCTURAL ENGINEER.

7. CLEAR COVER OF REINFORCEMENT SHALL BE AS FOLLOWS:

   - UNFORMED SURFACES IN CONTACT WITH EARTH 3"
   - FORMED SURFACES IN CONTACT WITH EARTH 2"
   - COLUMNS AND BEAMS 1-½"
   - FORMED EXTERIOR WALL SURFACES 2"
   - FORMED INTERIOR WALL SURFACES 1½"
   - SLABS 1½"

ROUGH CARPENTRY

1. ALL LUMBER SHALL BE FREE OF WARPING, CRACKS, CHECKS, OR OTHER IMPERFECTIONS DELETERIOUS TO THE INTENDED USE. ALL MEMBERS SHALL BE INSTALLED TRUE, SQUARE, AND/OR PLUMB AND SHALL FULLY BEAR ON ALL SUPPORTS OR CONNECTORS.

2. SAWN LUMBER SHALL BE S-DRY AND GRADED PER WCLIB OR WWPA RULES. LUMBER GRADES SHALL BE AS FOLLOWS, UON:

   - JOISTS, BEAMS, POSTS: NO. 1 DOUGLAS FIR
   - STUDS: NO. 1 DOUGLAS FIR

3. PLATES, LEDGERS ON CONC. OR MASONRY: NO. 1 PRESSURE TREATED D.F. PRESSURE TREATMENT TO BE ALKALINE COPPER QUAT (ACQ) ONLY.

4. GLUE-LAMINATED LUMBER SHALL CONFORM TO AITC STANDARD PS56 AND AITC STANDARD SPECIFICATIONS 101 THROUGH 120. ALL GLULAMS SHALL BE CBC COMBINATION 24F-V8.

5. ENGINEERED WOOD PRODUCTS SHALL BE MANUFACTURED BY i-LEVEL PER THE FOLLOWING GRADES:

   - PSL BEAM: 2.0E
   - PSL COLUMNS: 1.8E
   - LVL BEAMS & RAFTERS: 1.9E

6. GLULAMS SHALL CONFORM TO CBC COMBINATION 24F-V8, UON.

7. PLYWOOD SHEATHING SHALL CONFORM TO APA STANDARD PS-1 (CURRENT REVISION) 5 PLY AND SHALL USE EXTERIOR GLUE. ALL PLYWOOD SHALL BE LAID PERPENDICULAR TO SUPPORTS UNLESS OTHERWISE NOTED. IF EXPOSED TO EXTERIOR OR UNCONDITIONED SPACE, PLYWOOD SHALL BE MARINE GRADE PRESSURE TREATED.

8. UNLESS OTHERWISE NOTED, ALL NAILING OF FRAMING SHALL BE PER CBC TABLE 2304.9.1. ALL NAILS SHALL BE GALVANIZED OR STAINLESS STEEL COMMON NAILS, SEE NOTE 9 FOR ADDITIONAL INFO. EQUIVALENT PNEUMATICALLY DRIVEN NAILS MAY BE USED IF FASTENER MANUFACTURER PROVIDES AN ICBO APPROVED REPORT ACCORDING TO RESEARCH RECOMMENDATION REPORT #2403. FASTENERS SUBSTITUTED SHALL BE OF EQUAL OR GREATER LATERAL AND WITHDRAWAL STRENGTH AS THE COMMON NAIL SPECIFIED.

9. FOR EXTERIOR CONDITIONS (INCLUDING CRAWL SPACES, CRIPPLE WALLS, DECKS, ETC.) AND WHEN IN CONTACT W/ PRESSURE TREATED LUMBER, ALL BOLTS, LAGS, SCREWS, NAILS, & OTHER STEEL HARDWARE SHALL BE STAINLESS STEEL, UON. SEE CURRENT SIMPSON CATALOG FOR FURTHER REQUIREMENTS.

10. FOR INTERIOR CONDITIONED SPACE, ALL BOLTS SHALL BE HOT-DIP GALVANIZED AND SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1 WITH MATCHING WASHERS AND NUTS UNLESS OTHERWISE NOTED. DRILLED HOLES FOR BOLTS SHALL BE 1/32 TO 1/16 GREATER DIAMETER THAN THE BOLT INSTALLED. BOLTS SHALL BE INSTALLED WITHOUT DRIVING AND SHALL BE SNUGLY TIGHTENED.

11. CLEARANCE HOLES: SIZE AND DEPTH OF UNTHREADED SHANK.

12. LEAD HOLES: 75% OF THE SIZE AND 100% OF THE DEPTH OF THE THREADED SHANK.

13. LAG BOLTS SHALL BE INSTALLED BY TURNING WITH A WRENCH. UNDER NO CIRCUMSTANCES SHALL A LAG BOLT BE INSTALLED BY DRIVING WITH A HAMMER.

14. CONNECTION HARDWARE SHOWN ON THE PLANS SHALL BE SIMPSON STRONG-TIE OR AN APPROVED EQUIVALENT. ALL HARDWARE SHALL BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS AND WITH ALL NAILS OR OTHER FASTENERS. SEE ABOVE FOR MATERIAL/COATING REQUIREMENTS.

15. WHERE POSTS OR MULTIPLE STUDS UNDER BEAMS OR HEADERS ARE CALLED FOR ON DRAWINGS, THOSE POSTS OR MULTIPLE HEADERS SHALL BE CARRIED TO THE FOUNDATION.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", LATEST EDITIONS.

2. UNLESS OTHERWISE NOTED, STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

3. ALL EXPOSED STEEL AND FASTENERS TO BE STAINLESS STEEL, UON.

4. WIDE FLANGE SHAPES ASTM A992, GR. 50
   ANGLES, CHANNELS, PLATES & BARS ASTM A36
   STRUCTURAL TUBING (HSS) ASTM A500, GR. B
   PIPES (HSS) ASTM A500, GR. B
   BOLTS AND ANCHOR BOLTS ASTM A307
   HIGH-STRENGTH BOLTS ASTM A325

5. BOLT HOLES SHALL BE OF DIAMETER 1/16" LARGER THAN THE BOLT SIZE, UNLESS OTHERWISE NOTED.

6. ALL WELDING SHALL BE PERFORMED BY WELDERS CERTIFIED FOR THE WELDS TO BE MADE AND SHALL CONFORM TO AWS D1.1, LATEST EDITION. ALL WELDING ELECTRODES SHALL BE E70XX.

7. WELD FILLER METAL SHALL HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LBS AT MINUS 20 DEGREES F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURER CERTIFICATION.

8. SHEAR STUDS SHALL BE AWS D1.1 HEADED SHEAR CONNECTOR STUDS.

9. MATERIALS CERTIFICATIONS AND SHOP DRAWINGS FOR STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

STRUCTURAL OBSERVATION

PER SECTION 1709 OF THE CALIFORNIA BUILDING CODE, THE OWNER SHALL EMPLOY THE ARCHITECT OR ENGINEER OF RECORD TO PERFORM STRUCTURAL OBSERVATION OF THE STRUCTURAL WORK FOR GENERAL CONFORMANCE WITH THE STRUCTURAL PLANS. NOTE THAT THIS REQUIREMENT DOES NOT WAIVE RESPONSIBILITY OF THE LOCAL JURISDICTION TO PROVIDE THOROUGH INSPECTION OF THE WORK.

SPECIAL INSPECTION

EMPLOYMENT OF SPECIAL INSPECTORS IS THE DIRECT RESPONSIBILITY OF THE OWNER OR OWNER'S REPRESENTATIVE. SPECIAL INSPECTION SHALL BE ONE OF THE FOLLOWING PRESCRIBED IN CHAPTER 17 OF THE CALIFORNIA BUILDING CODE. THE NAME OF THE SPECIAL INSPECTOR SHALL BE FURNISHED TO THE DBI DISTRICT INSPECTOR PRIOR TO THE START OF WORK FOR WHICH SPECIAL INSPECTION IS REQUIRED.

SPECIAL INSPECTION AGENCY:

NAME: _____

AGENCY: _____

THE SPECIAL INSPECTOR(S) QUALIFICATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL 14 DAYS PRIOR TO COMMENCEMENT OF THE WORK. NO WORK REQUIRING SPECIAL INSPECTION SHALL BE PERFORMED w/o THE EXPRESS WRITTEN APPROVAL OF THE SPECIAL INSPECTOR BY THE STRUCTURAL ENGINEER.

SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING WORK:

INSPECTION TASK	Special Inspection by: FSE	Special Inspection by: Inspection Agency/ Other	Structural Observation by: FSE
1. Concrete Placement & Sampling			
A. Spread Footings			
B. Drilled Piers		X	
C. Retaining Walls		X	
D. Grade Beams			
E. Columns/beams			
2. Reinforcing steel and pre-stressing tendons			
A. Placement inspection		X	X
B. Stressing and grouting of tendons			
3. Bolts installed in concrete (wet set bolts)			X
4. Bolts installed in existing concrete:			
Epoxy grouted			
Pull/torque tests for SFBC Sec 1607C & 1615C			
5. Special moment-resisting concrete frame			
6. Structural masonry			
7. Shotcrete			
8. Foundations			
Shallow footing excavation		X	
Piling, drilled piers and caissons		X	
Others _____			
9. Structural welding:			
A. Periodic visual inspection			
Single pass fillet welds ≤ 5/16"		X	X
Steel deck			
Welded studs			
Cold formed studs and joists			
Stair and railing systems			
B. Continuous visual inspection and NDT except as noted above and on plans and details			
All welds except as listed above			
Reinforcing steel			
Moment-resisting frames			
Others _____			
10. High-strength bolting			
11. Reinforced gypsum concrete			
12. Insulating concrete fill			
13. Sprayed-on fireproofing			
14. Special grading, excavation and filling (Geo. Engineered)			
15. Smoke-control system			
16. Demolition			
17. Exterior Facing			
18. Retrofit of unreinforced masonry buildings:			
Testing of mortar quality and shear tests			
Inspection of repointing operations			
Installation inspection of new shear bolts			
Pre-installation inspection for new shear bolts			
Pull/torque tests per SFBC Sec 1707C & 1615C			
19. Shear walls, proprietary shear walls, and shear diaphragms	X		X
20. Holdowns	X		X
21. Special cases:			
Shoring			
Underpinning			
Others:			
22. Crane safety (Apply to operation of tower cranes on Sec 1701.8)			
23. Others: As recommended by "Professional of record"			

CITY/COUNTY INSPECTOR:

NAME: _____

TEL: _____

THE CITY/COUNTY INSPECTOR SHALL NOT GIVE APPROVAL FOR NOR SHALL THE CONTRACTOR PROCEED WITH ANY WORK WHICH WILL CLOSE ACCESS TO ANY WORK REQUIRING SPECIAL INSPECTION PRIOR TO RECEIVING WRITTEN CONFIRMATION THAT THE SPECIAL INSPECTIONS HAVE BEEN PERFORMED AND THE INSPECTED WORK APPROVED BY THE SPECIAL INSPECTOR LISTED.

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

REVISIONS DATE

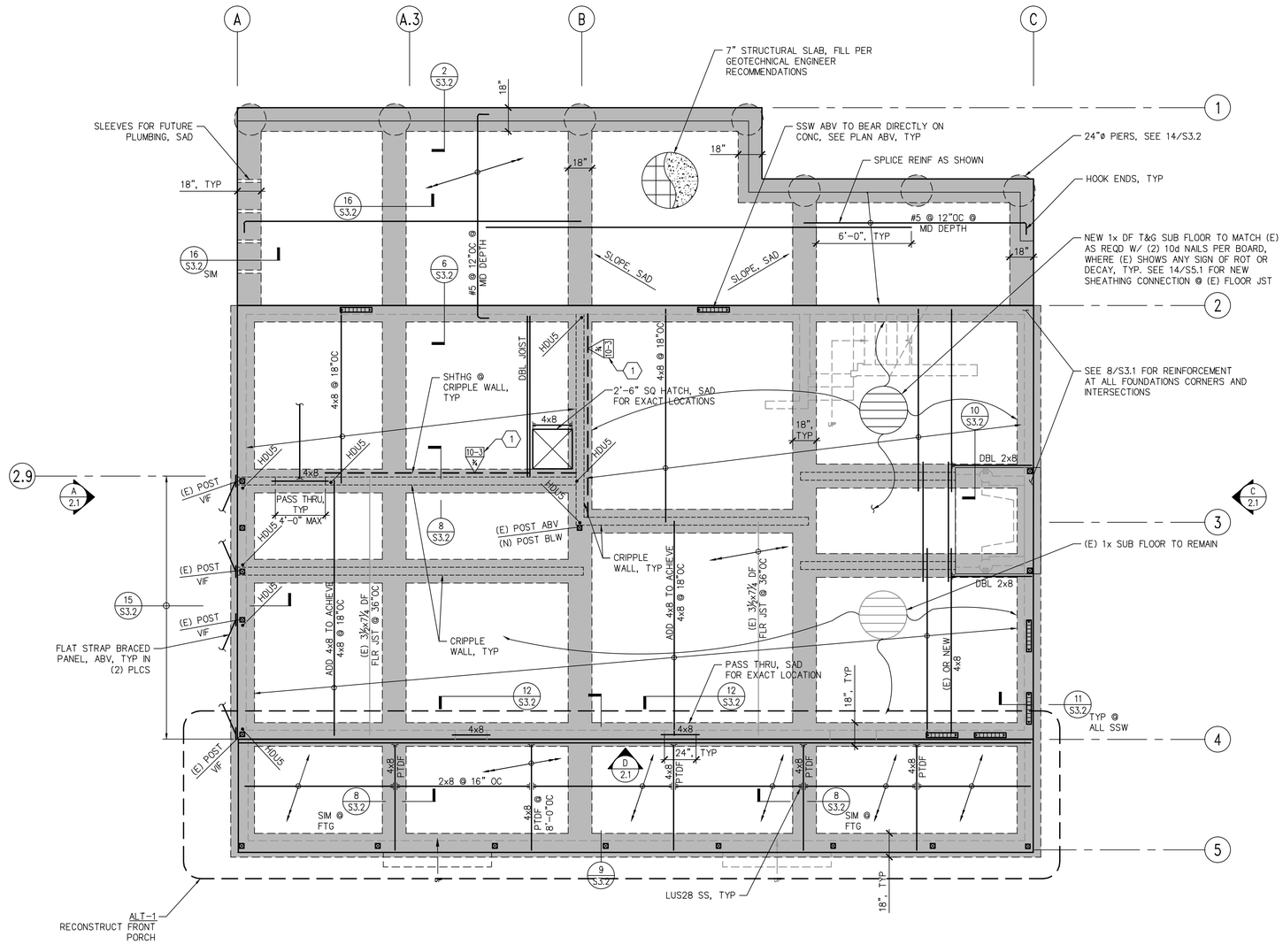
NAVARRO INN
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L. ROOD
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

REVISED 100% CD SET

GENERAL NOTES & ABBREV

DATE May 16, 2011
SCALE AS NOTED
DRAWN FSE
JOB 08-37
SHEET S0.1
OF 10 SHEETS



A FOUNDATION/ FIRST FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

PROJECT & REAL NORTH

LEGEND

- POST BELOW FRAMING LEVEL (UON) CARRY TO FDN UON (SEE PLAN ABV FOR TYPE & SIZE)
- STEEL COLUMN (SEE PLAN ABV FOR TYPE & SIZE)
- HORIZ HOLDOWN
- HORIZ STRAP
- VERT HOLD-DOWN/ STRAP
- (N) WALL ABOVE FF
- (N) WALL BELOW FF
- (N) CRIPLE WALL
- (E) WALL ABOVE FF
- (E) WALL BELOW FF
- SHEARWALL LENGTH / NAILING SCHEDULE / PLY THICKNESS
- SINGLE/DBL SHEARWALL
- SINGLE/DBL SHEARWALL BELOW
- OPENING
- BEAM HANGER
- WALL HEADER
- SECTION: SECTION NUMBER / SECTION SHEET
- DIRECTION OF JOIST LAYOUT
- DOUBLE JOIST

KEY NOTES LEGEND:

1. STRUCTURAL PLYWOOD @ CRAWL SPACE TO BE PRESSURE TREATED W/ ONLY FACTORY TREATED EDGE DOWN, ALL CUT EDGES TO BE TREATED W/ FIELD APPLIED COPPER BASED PRESERVATIVE OR COPPER FLASHING. SEE ROUGH CARPENTERS NOTE #7, SHEET S0.1 FOR ADDITIONAL INFORMATION.

NOTES:

1. CONTRACTOR TO OBSERVE AND INSPECT (E) FRAMING FOR ANY SIGNS OF ROT OR DECAY. WHERE ANY SIGNS OF ROT OR DECAY ARE OBSERVED IN ANY PART OF FRAMING MEM, THAT MEM SHALL BE REPLACED IN KIND WITH DF SELECT STRUCT. NOTIFY ARCHITECT AND ENGINEER PRIOR TO INSTALLATION.

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665 THIRD ST, STE 333
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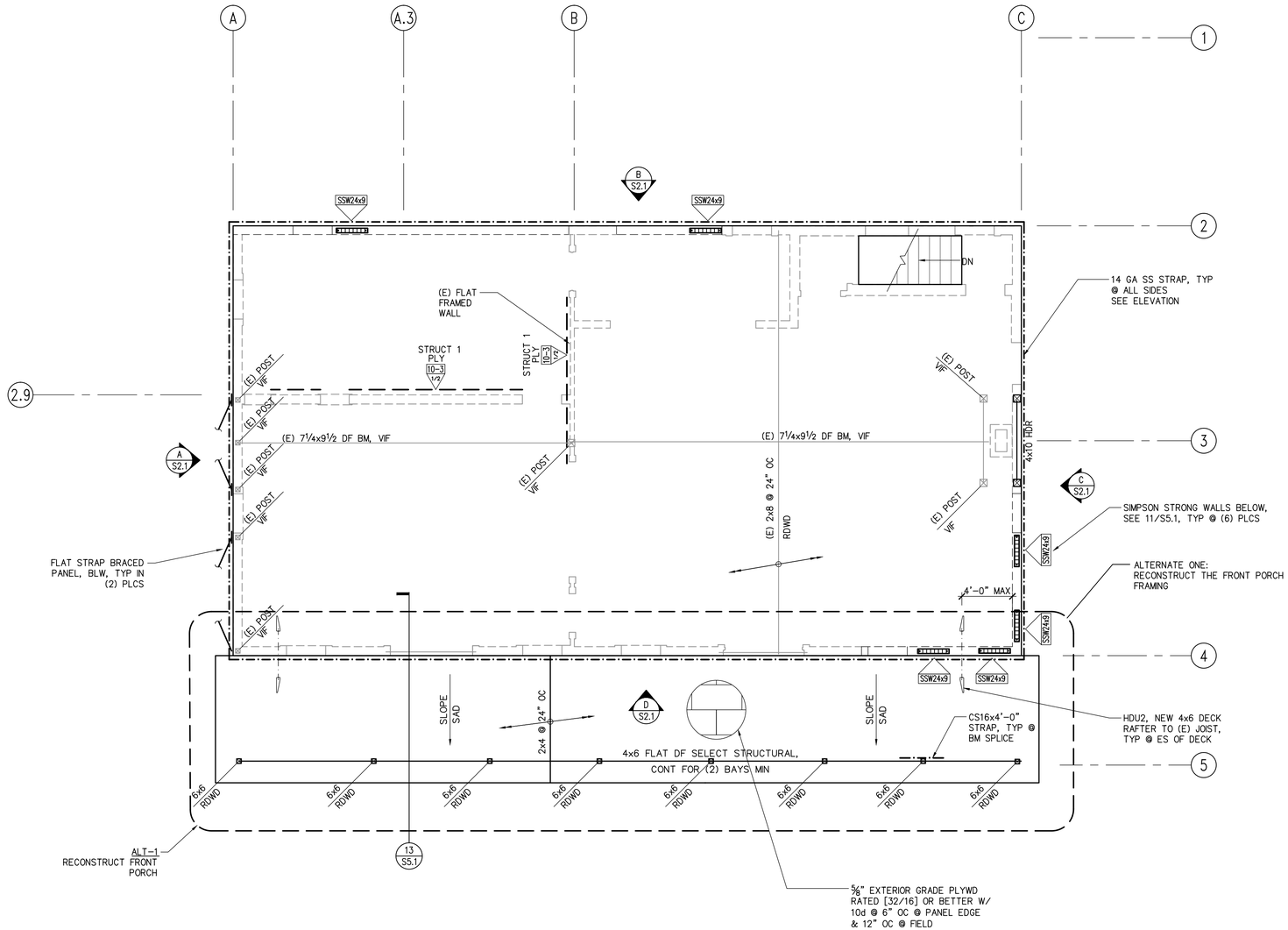
FOUNDATION/ FIRST FLOOR FRAMING PLAN

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S1.1
OF	10 SHEETS



A SECOND FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

PROJECT & REAL NORTH

LEGEND

- 6x6 — POST BELOW FRAMING LEVEL (UON) CARRY TO FDN UON (SEE PLAN ABV FOR TYPE & SIZE)
- STEEL COLUMN (SEE PLAN ABV FOR TYPE & SIZE)
- HORIZ HOLDOWN
- HORIZ STRAP
- PHD2, CMSTC16 — VERT HOLD-DOWN/ STRAP
- (N) WALL ABOVE FF
- (N) WALL BELOW FF
- (E) WALL ABOVE FF
- (E) WALL BELOW FF
- 3'-0" / 8-4 / 10-3 — SHEARWALL LENGTH, NAILING SCHEDULE, PLY THICKNESS
- SINGLE/DBL SHEARWALL
- SINGLE/DBL SHEARWALL BELOW
- OPENING
- BEAM HANGER
- 4x8 — WALL HEADER
- 5 / S3.3 — SECTION: SECTION NUMBER, SECTION SHEET
- 2x6 @16" — DIRECTION OF JOIST LAYOUT
- 2x6 DBL — DOUBLE JOIST

NOTES:

1. CONTRACTOR TO OBSERVE AND INSPECT (E) FRAMING FOR ANY SIGNS OF ROT OR DECAY. WHERE ANY SIGNS OF ROT OR DECAY ARE OBSERVED IN ANY PART OF FRAMING MEM, THAT MEM SHALL BE REPLACED IN KIND WITH DF SELECT STRUCT. NOTIFY ARCHITECT AND ENGINEER PRIOR TO INSTALLATION.

SPECIAL INSPECTION

1. THE SPECIAL INSPECTOR SHALL BE APPROVED IN WRITING BY ENGINEER AND HIRED BY THE OWNER A MINIMUM OF TWO WEEKS PRIOR TO COMMENCEMENT OF CONSTRUCTION

SHOP DRAWINGS

1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR STRUCTURAL STEEL TO THE ENGINEER FOR REVIEW AND APPROVAL A MINIMUM OF (4) WEEKS PRIOR TO FABRICATION AND TARGET DELIVERY DATE.

2. ALL APPROVED SHOP DRAWINGS SHALL BE KEPT ON-SITE AND BE MADE AVAILABLE TO THE SPECIAL INSPECTOR DURING THE INSPECTION OF RELATING STRUCTURAL ITEMS.

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

REVISIONS DATE

NAVARRO INN

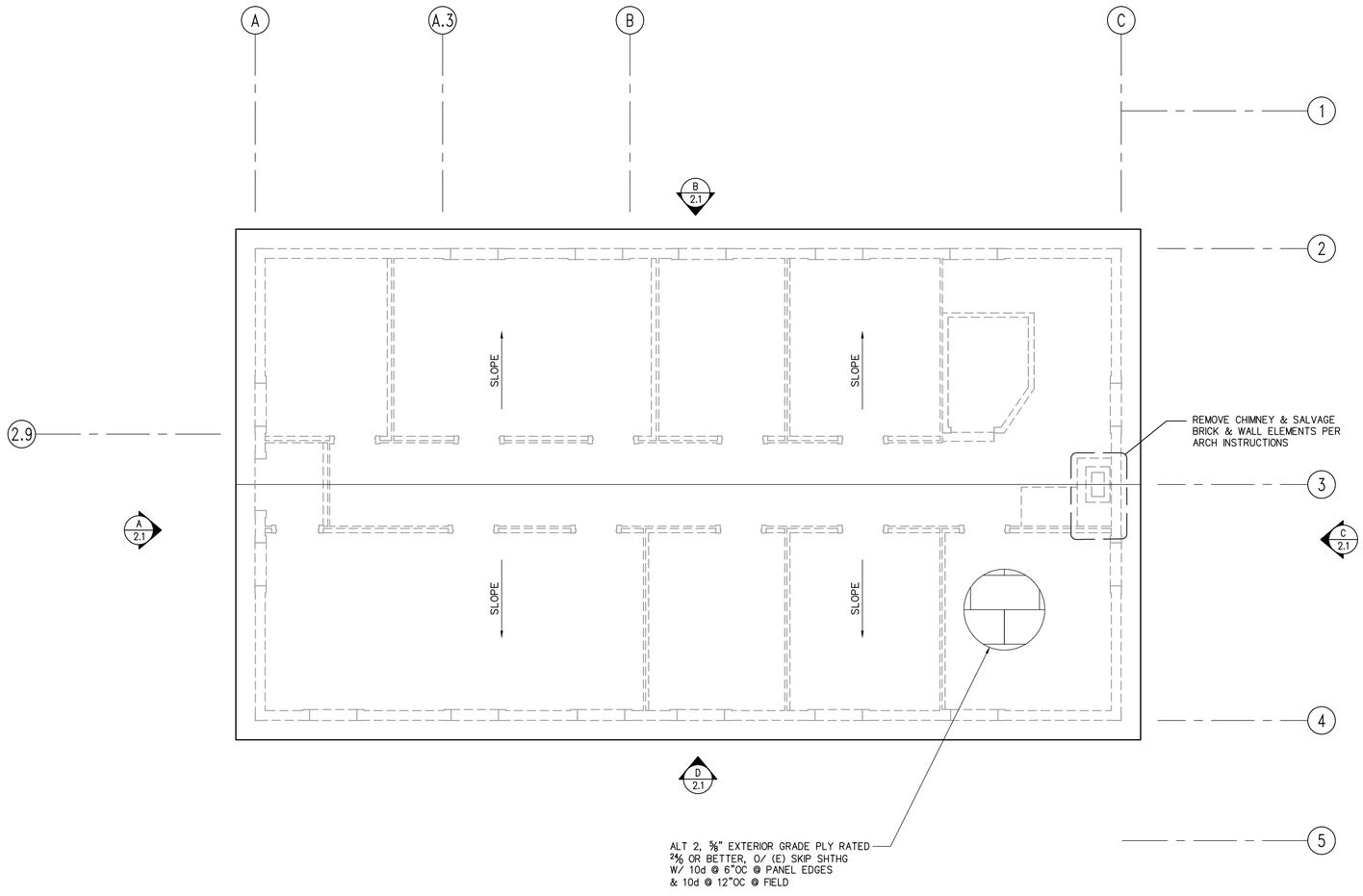
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L. BOOL
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

REVISED 100% CD SET

SECOND FLOOR FRAMING PLAN

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S1.2
OF	10 SHEETS



LEGEND

- 6x6 — POST BELOW FRAMING LEVEL (UON) CARRY TO FDN UON (SEE PLAN ABV FOR TYPE & SIZE)
- STEEL COLUMN (SEE PLAN ABV FOR TYPE & SIZE)
- HORIZ HOLDOWN
- HORIZ STRAP
- THD2 / CMST216 — VERT HOLD-DOWN/ STRAP
- (N) WALL ABOVE FF
- (N) WALL BELOW FF
- (E) WALL ABOVE FF
- (E) WALL BELOW FF
- 3'-0" — SHEARWALL LENGTH; 10-d — NAILING SCHEDULE; 7/8 — PLY THICKNESS
- SINGLE/DBL SHEARWALL
- SINGLE/DBL SHEARWALL BELOW
- OPENING
- BEAM HANGER
- 4x8 — WALL HEADER
- 5 / S3.3 — SECTION: SECTION NUMBER / SECTION SHEET
- 2x6 @16" — DIRECTION OF JOIST LAYOUT
- 2x6 DBL — DOUBLE JOIST

NOTES:

1. CONTRACTOR TO OBSERVE AND INSPECT (E) FRAMING FOR ANY SIGNS OF ROT OR DECAY. WHERE ANY SIGNS OF ROT OR DECAY ARE OBSERVED IN ANY PART OF FRAMING MEM, THAT MEM SHALL BE REPLACED IN KIND WITH DF SELECT STRUCT. NOTIFY ARCHITECT AND ENGINEER PRIOR TO INSTALLATION.

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1. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR TRUSSES AND STRUCTURAL STEEL TO THE ENGINEER FOR REVIEW AND APPROVAL A MINIMUM OF (4) WEEKS PRIOR TO FABRICATION AND TARGET DELIVERY DATE.

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A ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

PROJECT & REAL NORTH

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

REVISIONS DATE

NAVARRO INN

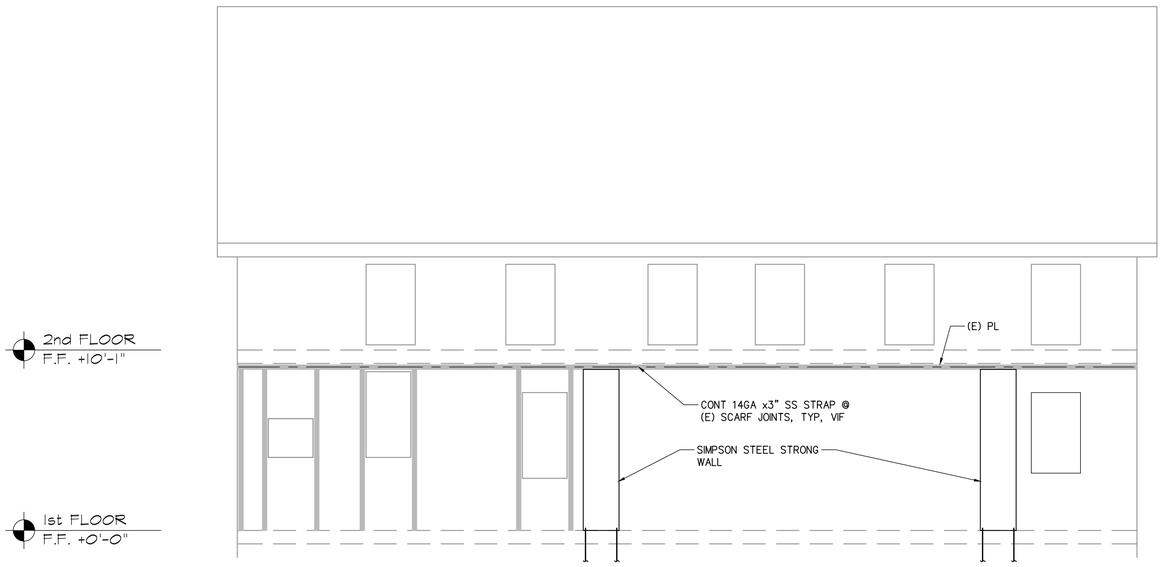
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER — ALEX L. ROOT — No. S 004492 — Exp. 12/31/12 — STRUCTURAL — STATE OF CALIFORNIA

REVISED 100% CD SET

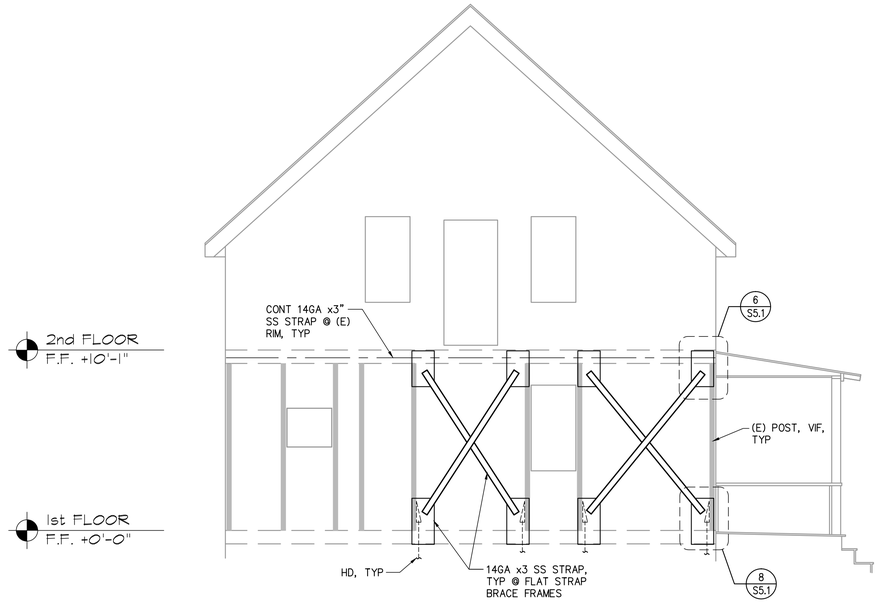
ROOF FRAMING PLAN

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S1.3
OF	10 SHEETS



B SOUTH ELEVATIONS

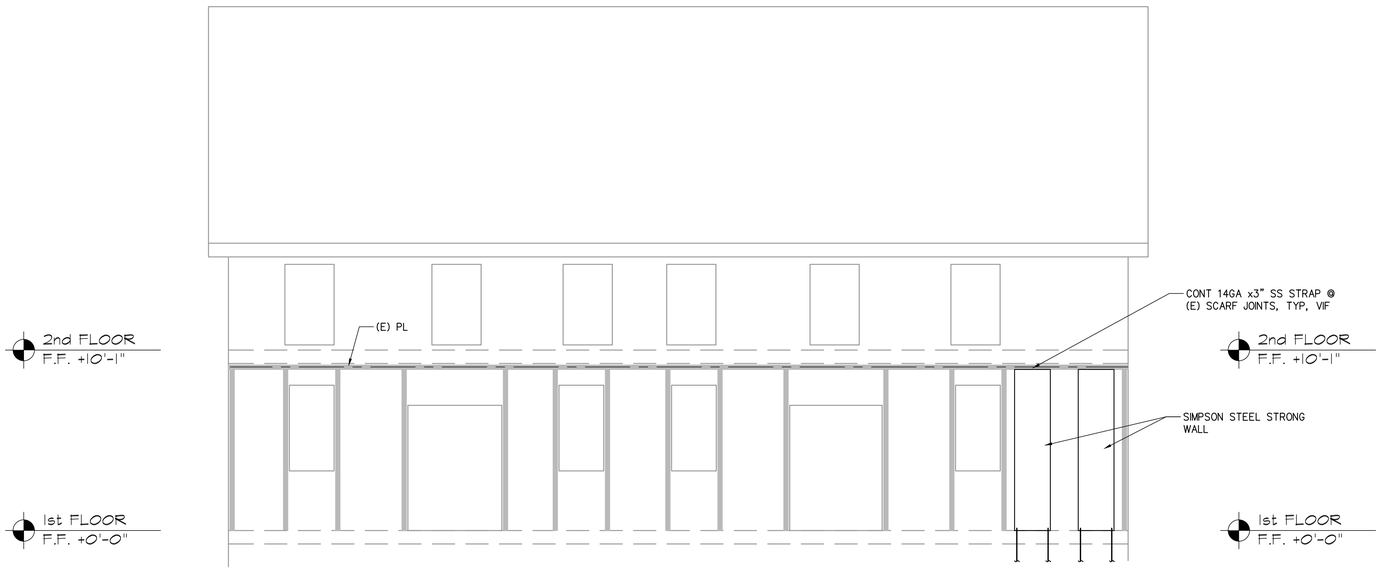
SCALE: 1/4" = 1'-0"



NOTE:
NOT ALL ELEMENTS
SHOWN FOR CLARITY

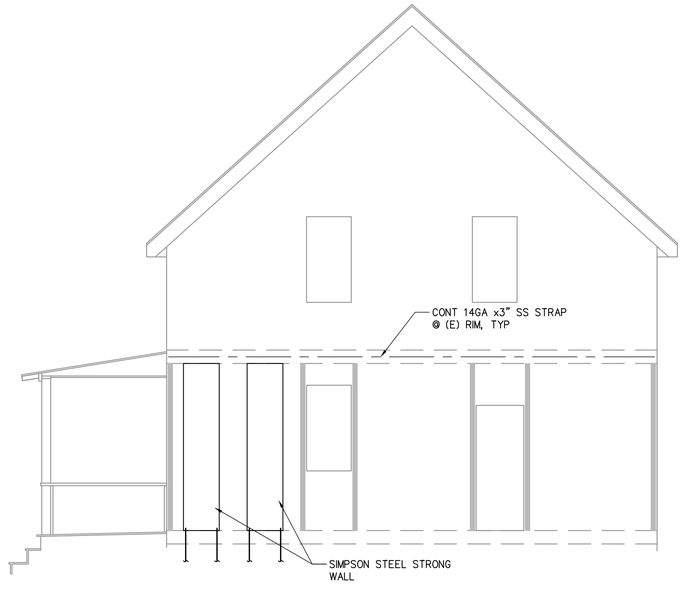
A EAST ELEVATIONS

SCALE: 1/4" = 1'-0"



D NORTH ELEVATIONS

SCALE: 1/4" = 1'-0"



C WEST ELEVATIONS

SCALE: 1/4" = 1'-0"

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STRUCTURAL ENGINEERING

665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
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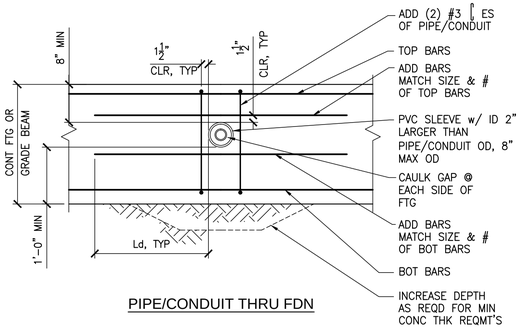
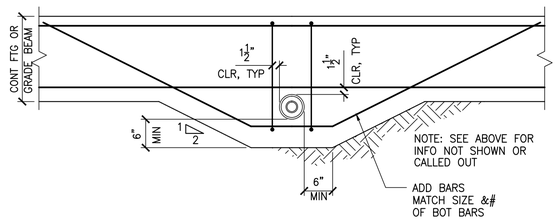
ELEVATIONS

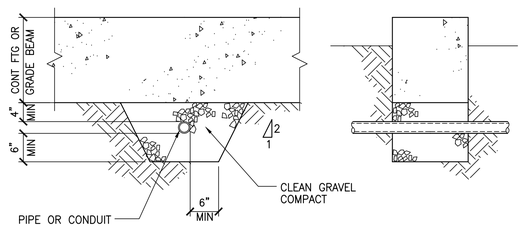
DATE May 16, 2011
SCALE AS NOTED
DRAWN FSE
JOB 08-37
SHEET
S2.1
OF 10 SHEETS



PIPE/CONDUIT THRU FDN



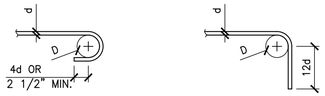
PIPE/CONDUIT BELOW BOT BARS



PIPE/CONDUIT BELOW FDN

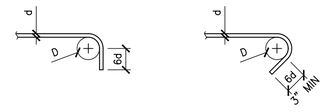
11 PIPES & CONDUITS THRU & BELOW CONT FTGS

SCALE: 3/4"= 1'-0" 00BP–FN1–11



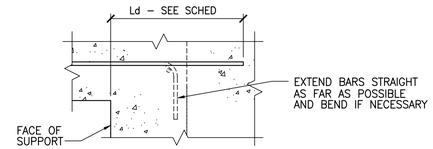
180° BEND 90° BEND

PRINCIPAL REINFORCEMENT		
BAR GRADE	BAR SIZE	MIN. BEND DIA. 'D'
ALL GRADES OF REIN–FORCEMENT	#3 THRU #8	6d
	#9 THRU #11	8d
	#14 THRU #18	10d
GRADE 40*	#3 THRU #11	5d



90° BEND 135° BEND

STIRRUPS AND TIE REINFORCEMENT	
BAR SIZE	MIN. BEND DIA. 'D'
#3 THRU #5	4d
ALL OTHER BARS	SEE TABLE ABOVE



REBAR EXTENSION BEYOND FACE OF SUPPORTS

6 BAR BENDS & HOOKS

SCALE: 3/4"= 1'-0" 00BP–FN1–05

BASIC DEVELOPMENT LENGTH (Ld) (IN)						
	f'c=2500 PSI		f'c=4000 PSI		f'c=5000 PSI	
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	24	18	21	15	21	15
#4	32	24	26	19	23	17
#5	40	30	32	24	29	21
#6	49	36	38	28	34	25
#7	69	53	54	43	49	36
#8	78	60	62	48	56	43
#9	88	68	70	54	63	48

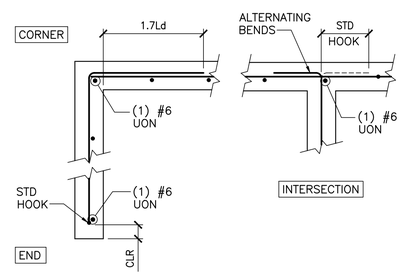
BAR SPLICE LENGTH SCHEDULE (Ls) (IN)						
	f'c=2500 PSI		f'c=4000 PSI		f'c=5000 PSI	
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	31	23	28	20	28	20
#4	42	31	34	24	30	22
#5	52	39	42	31	38	27
#6	64	47	50	37	45	33
#7	90	69	70	54	63	49
#8	101	78	81	63	73	56
#9	114	88	91	71	82	63

NOTES:

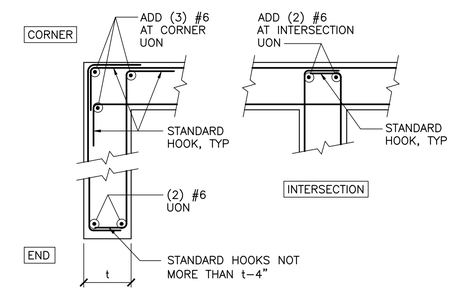
1. THE SCHEDULES SHOWN APPLY TO NORMAL WEIGHT CONCRETE. FOR LIGHT WEIGHT CONCRETE INCREASE LENGTHS BY 30%.
2. TOP BARS ARE HORIZONTAL BARS PLACED WHERE MORE THAN 12" OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.
3. STAGGER SPLICES WHEREVER POSSIBLE. WHERE CLASS "A" SPLICE IS SPECIFICALLY CALLED FOR IN THE DRAWINGS, STAGGER AT LEAST 50% OF THE BARS. CLASS "A" SPLICE LENGTH IS 80% OF THE CLASS "B" SPLICE SHOWN IN THE SCHEDULE.
4. WHEN SPLICING BARS OF DIFFERENT SIZE, THE LAP SHALL BE BASED ON THE LARGER OF:
   a) THE SPLICE LENGTH OF THE SMALLER BAR
   b) THE DEVELOPMENT LENGTH OF THE LARGER BAR.
5. LAP LENGTH SPECIFICALLY DETAILED ON THE DRAWINGS SHALL GOVERN IN LIEU OF THE LAP LENGTH IN THE SCHEDULE.
6. WHERE CLEAR SPACING OF BARS IS GREATER THAN 2 x BAR DIA OR CLEAR COVER IS GREATER THAN BAR DIA, MULTIPLY DEV OR SPLICE LEN BY 1.5.

2 REBAR DEVELOPMENT & SPLICE LENGTHS

SCALE: 3/4"= 1'-0" 00BP–FN1–02



SINGLE CURTAIN



DOUBLE CURTAIN

8 REBAR @ ENDS & INTERSECTIONS OF WALLS AND/OR FTGS

SCALE: 3/4"= 1'-0" 00BP–FN1–06

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST, STE 33
SAN FRANCISCO, CA 9410
TEL: 415.543.016
FAX: 415.543.016

REVISIONS DATE

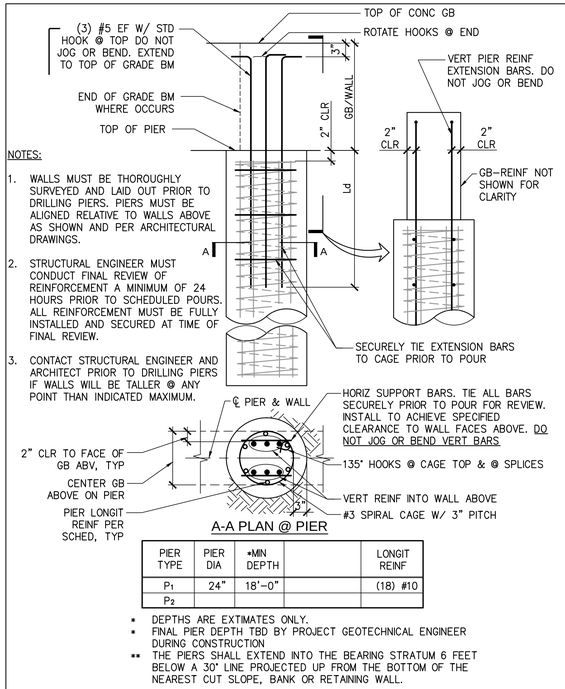
NAVARRO INN
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L. HOOD
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

REVISED 100% CD SET

TYP FOUNDATION DETAILS

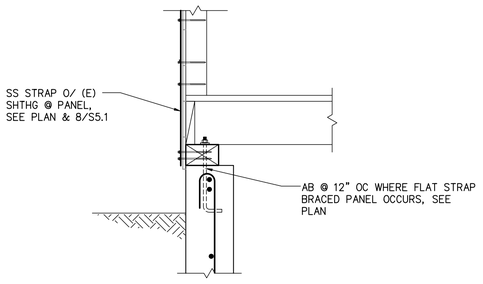
DATE May 16, 2011
SCALE AS NOTED
DRAWN FSE
JOB 08-37
SHEET S3.1
OF 10 SHEETS



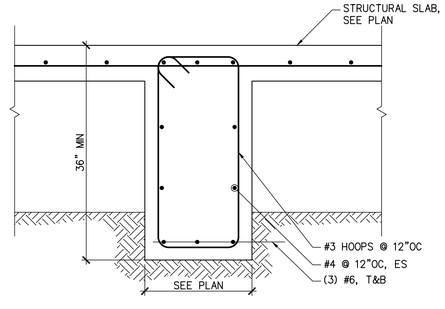
PIER TYPE	PIER DIA	*MIN DEPTH		LONGIT REINF
P1	24"	18'-0"		(18) #10
P2				

- DEPTHS ARE EXTIMATES ONLY.
- FINAL PIER DEPTH TBD BY PROJECT GEOTECHNICAL ENGINEER DURING CONSTRUCTION
- THE PIERS SHALL EXTEND INTO THE BEARING STRATUM 6 FEET BELOW A 30° LINE PROJECTED UP FROM THE BOTTOM OF THE NEAREST CUT SLOPE, BANK OR RETAINING WALL.

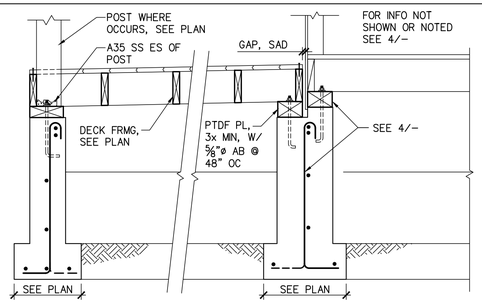
14 DRILLED CONCRETE PIERS
SCALE: 3/4"= 1'-0"



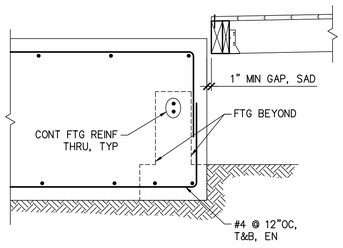
15 FOUNDATION @ FLAT STRAP BRACED PANEL
SCALE: 1"= 1'-0"



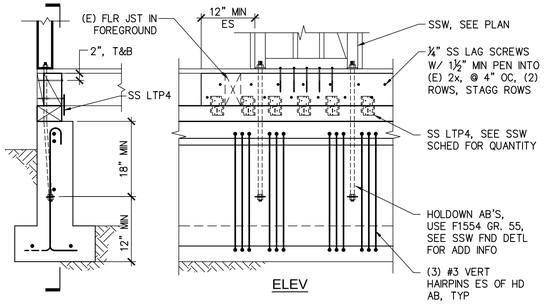
16 GRADE BEAM @ STRUCTURAL SLAB
SCALE: 1"= 1'-0"



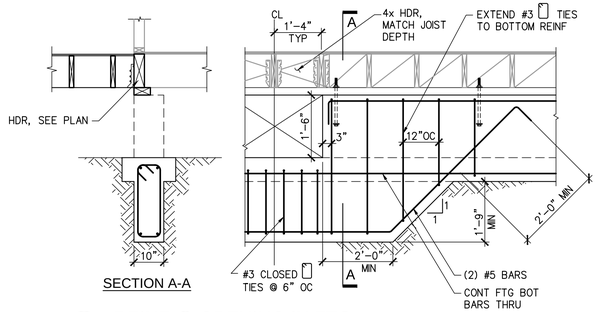
9 FOOTING AT ENTRY
SCALE: 3/4"= 1'-0"



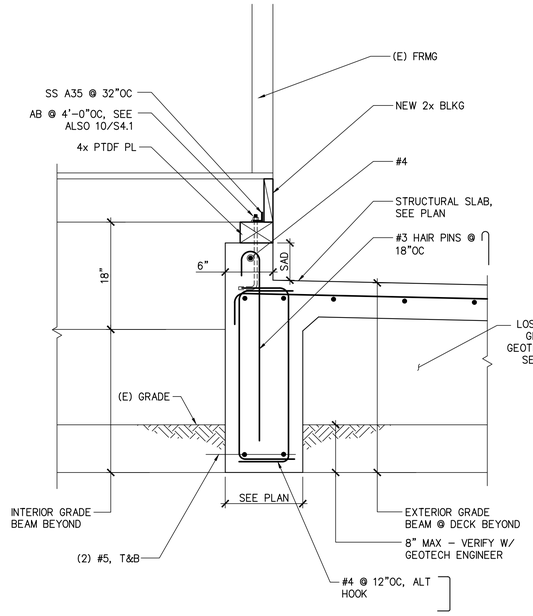
10 FOOTING AT ENTRY
SCALE: 3/4"= 1'-0"



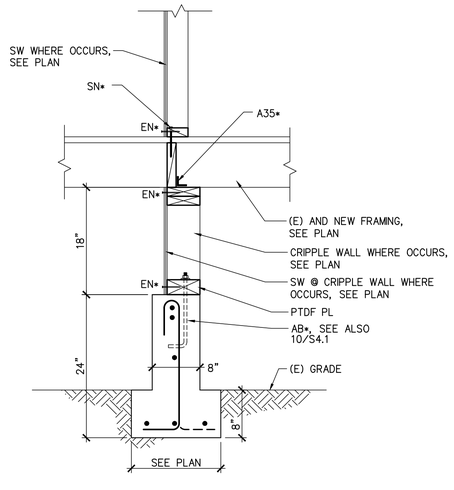
11 SSW @ FOUNDTATION
SCALE: 3/4"= 1'-0"



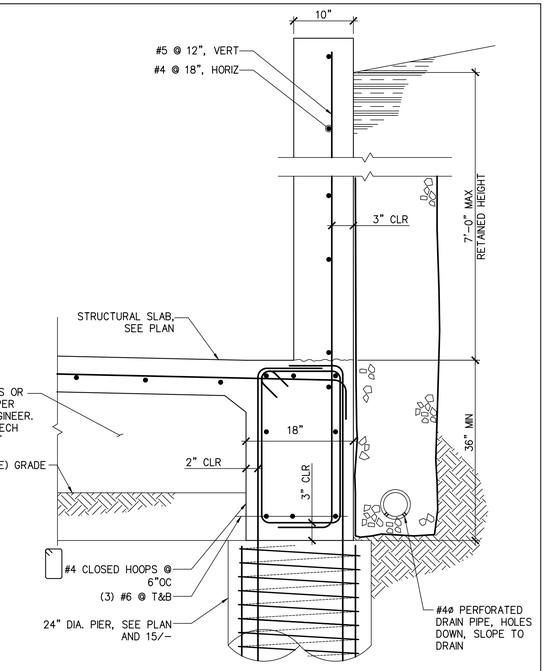
12 REINF @ OPENING IN FTG
SCALE: 1/2"= 1'-0"



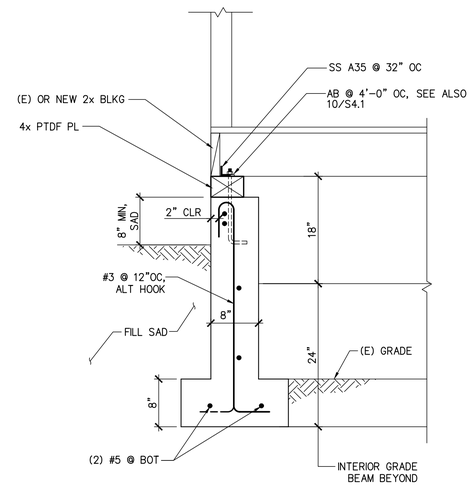
6 EXTERIOR GB @ SOUTH SIDE OF HOUSE
SCALE: 1"= 1'-0"



8 INTERIOR GRADE BEAM
SCALE: 1"= 1'-0"



2 RETAINING WALL
SCALE: 1"= 1'-0"



4 EXTERIOR GB
SCALE: 1"= 1'-0"

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

REVISIONS DATE

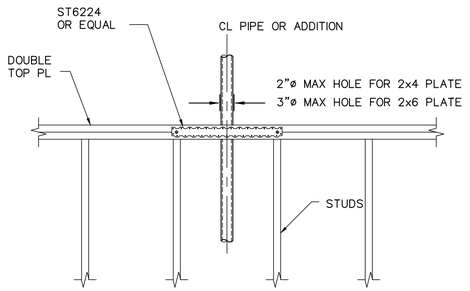
NAVARRO INN
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER ALEX L. ROOD No. S 004492 Exp. 12/31/12 STRUCTURAL STATE OF CALIFORNIA

REVISED 100% CD SET

FOUNDATION DETAILS

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S3.2
OF	10 SHEETS



13 SPLICE @ PIPE OR ADDITION

SCALE: NTS 00BP-WD2-13

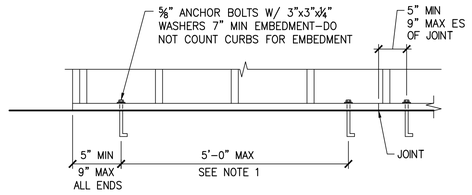
JOIST/BEAM HANGER SCHEDULE	
JOIST OR BEAM	HANGER TYPE
2X ROOF JOIST	U
2X FLOOR JOIST	U
TJI JOIST	IUT
DBL TJI JOIST	IUT
4x	HU OR B[1]
6x	HU OR B[1]
GLULAMS, LVL, PSL	GB OR HW[1]

HANGERS MANUFACTURED BY SIMPSON OR APPROVED EQUAL. INSTALL PER MANUF. INSTRUCTIONS UON. USE HANGERS AS SCHEDULED, UON.

USE LARGEST HANGER FOR MEMBER SIZE. USE FULL NAILING.

9 TYP HANGER SCHEDULE

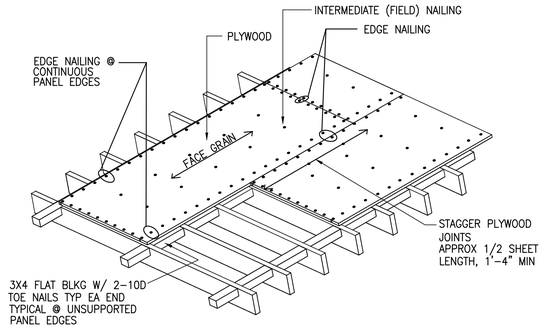
SCALE: NTS 00BP-WD2-09



ELEV

NOTES:

1. SEE PLAN & SHEARWALL SCHEDULE FOR ADDITIONAL SILL PLATE BOLTING REQUIREMENTS.
2. CONTRACTOR SHALL EXPOSE AND EXAMINE (E) WALLS AND ADD EPOXY ANCHOR BOLTS TO PROVIDE MINIMUM SPECIFIED BOLTING WHERE ABSENT. WHERE REQUIRED, SPECIAL INSPECTION IS REQUIRED FOR ALL EPOXY ANCHOR BOLTS. TORQUE TESTING MAY BE REQUIRED (PER LOCAL REQUIREMENTS) AS WELL.



TYPICAL FLOOR/ROOF SHEATHING

NOTES:

1. EDGE DISTANCE OF NAILS IN JOISTS AND PLYWOOD SHEETS SHALL BE 3/8" MINIMUM.
2. PROVIDE EDGE BLOCKING AS SHOWN ABOVE WHERE INDICATED ON PLANS.

EXTERIOR WALLS									
"W" SPAN WIDTH	LEVELS SUPPORTED								
	ROOF ONLY			ROOF + 1 FLR			ROOF + 2 FLRS		
	HDR	BRNG STUDS	KING STUDS	HDR	BRNG STUDS	KING STUDS	HDR	BRNG STUDS	KING STUDS
4'	4x6	2x4	2x4	(2)2x8	2x4	2x4	4x12	(2)2x4	2x4
6'	4x8	2x4	(2)2x4	4x12	(2)2x4	(2)2x4	4x10P	(2)2x4	(2)2x4
8'	4x10	2x4	(2)2x4	4x10P	(2)2x4	(2)2x4	4x12P	(3)2x4	(2)2x4

INTERIOR WALLS									
"W" SPAN WIDTH	LEVELS SUPPORTED								
	ROOF ONLY			ROOF + 1 FLR			ROOF + 2 FLRS		
	HDR	BRNG STUDS	KING STUDS	HDR	BRNG STUDS	KING STUDS	HDR	BRNG STUDS	KING STUDS
4'	4x6	2x4	2x4	4x12	(2)2x4	2x4	4x10P	(2)2x4	2x4
6'	4x8	(2)2x4	2x4	4x10P	(2)2x4	2x4	4x12P	(3)2x4	2x4
8'	4x12	(2)2x4	2x4	4x12P	(2)2x4	2x4	4x14P	(4)2x4	2x4
10'	*	(2)2x4	2x4	4x10P	(2)2x4	2x4	4x12P	(3)2x4	2x4
12'	*	(2)2x4	2x4	4x12P	(2)2x4	2x4	4x14P	(4)2x4	2x4

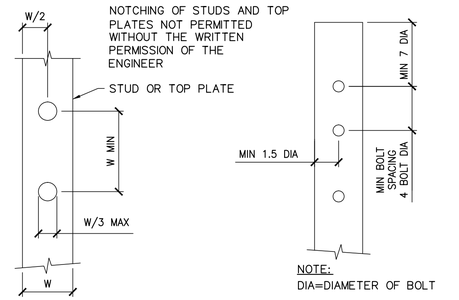
* SEE PLAN

NOTES:

1. MATCH STUD WIDTH
2. "P" DENOTES PSL, TYP.
3. SEE PLAN FOR SIZES. USE LARGER HEADER SIZE OF THAT SHOWN IN SCHEDULE AND THAT SHOWN ON PLAN.

2 HEADER SCHEDULE

SCALE: NTS 00BP-WD2-02



HOLES IN STUDS & TOP PLATES

NOTE:
DIA=DIAMETER OF BOLT

WOOD BOLT CLEARANCES

14 TYPICAL BOLT CLEARANCES

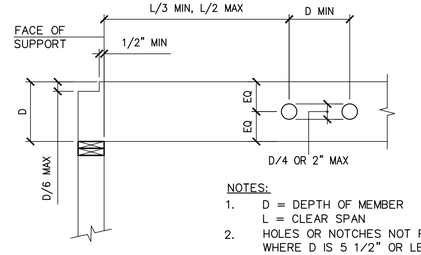
SCALE: NTS 00BP-WD2-14

10 SILL PLATE BOLTING

SCALE: 3/4"= 1'-0" 00BP-WD2-10

6 FLOOR/ROOF PLYWOOD NAILING

SCALE: NTS 00BP-WD2-06

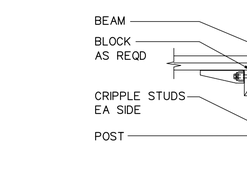


NOTES:

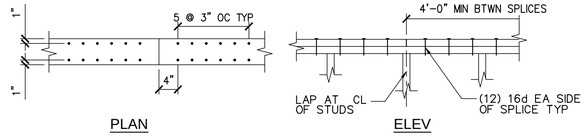
1. D = DEPTH OF MEMBER
   L = CLEAR SPAN
2. HOLES OR NOTCHES NOT PERMITTED WHERE D IS 5 1/2" OR LESS, OR IN BOTTOM OF JOIST OR BEAM
3. PREDRILL CORNERS OF NOTCHES
4. SEE MANUFACTURER'S REQUIREMENTS FOR TRUSS-JOIST PRODUCTS.

15 HOLES & NOTCHES BEAMS & JOISTS

SCALE: NTS 00BP-WD2-15

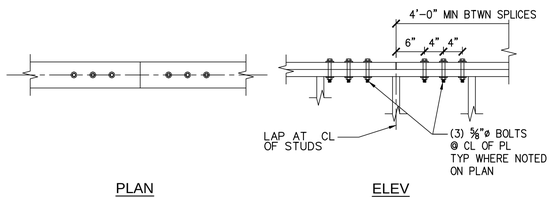


SPLICE AT BEAM



PLAN ELEV

NAILED SPLICE

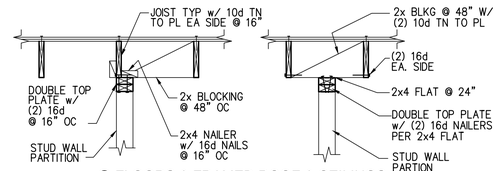


PLAN ELEV

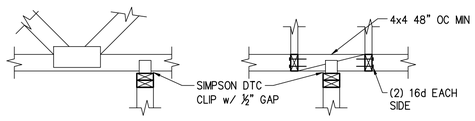
BOLTED SPLICE

12 TYP TOP PLATE SPLICE OPTIONS

SCALE: NTS 00BP-WD2-12



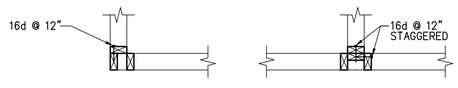
@ FLOORS & FRAMED ROOF & CEILINGS



@ TRUSS FLOORS & ROOFS

7 TYP NON-BEARING STUD WALL

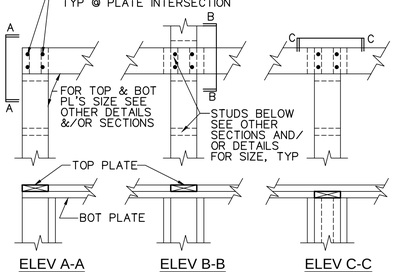
SCALE: NTS 00BP-WD2-07



@ CORNERS @ INTERSECTIONS

8 TYP STUD WALL CONNECTIONS

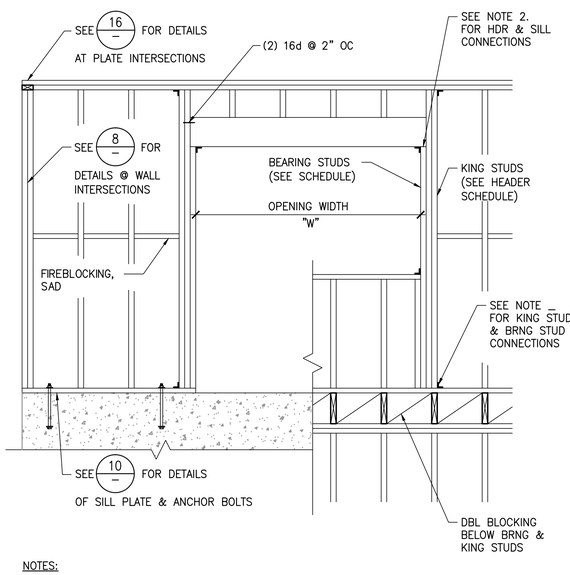
SCALE: NTS 00BP-WD2-08



ELEV A-A ELEV B-B ELEV C-C

16 TYP INTERSECTION @ PLATES

SCALE: NTS 00BP-WD2-16



NOTES:

1. ALL NAILING IS PER CBC TABLES 23-II-B-1 UNLESS NOTED OTHERWISE.
2. CONNECT HEADERS & SILLS TO BEARING STUDS: W ≤ 4'-0" TOENAIL/END NAIL PER 23-II-B-1 W> 4'-0" A34
3. CONNECT KING STUDS TO SILL & TOP PLATES: W ≤ 4'-0" TOENAIL/END NAIL PER 23-II-B-1 W > 4'-0" ADD A34 T&B

4 TYPICAL STUD WALL FRAMING ELEVATION

SCALE: NTS 00BP-WD2-04

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST, STE 333
SAN FRANCISCO, CA 9410
TEL: 415.543.016
FAX: 415.543.016

REVISIONS DATE

NAVARRO INN

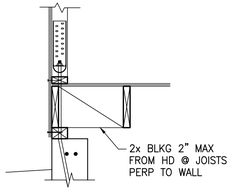
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L ROOD
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

REVISED 100% CD SET

TYPICAL FRAMING DETAILS I

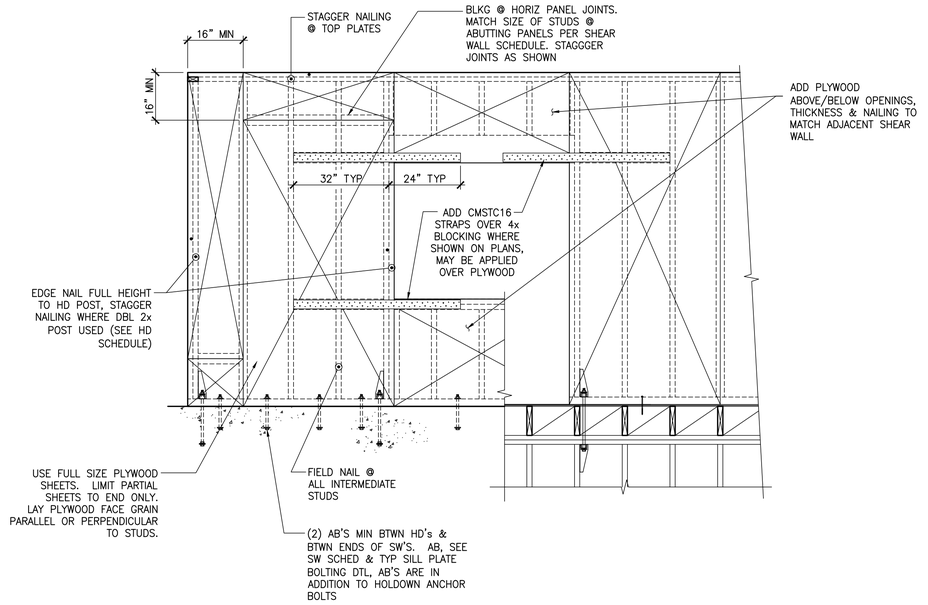
DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S4.1
OF	10 SHEETS



9 TYP HOLDOWN @ FLR O/FDN

SCALE: NTS

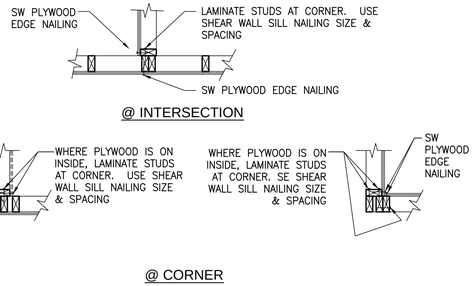
00BP-WD1-09



6 TYP SHEAR WALL FRAMING ELEVATION

SCALE: NTS

00BP-WD1-06



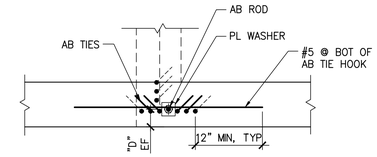
@ INTERSECTION

@ CORNER

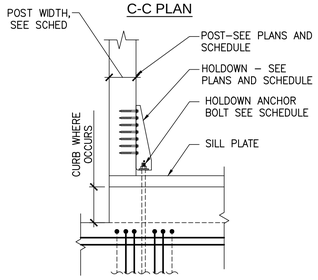
16 TYP SHEAR WALL CONNECTIONS

SCALE: NTS

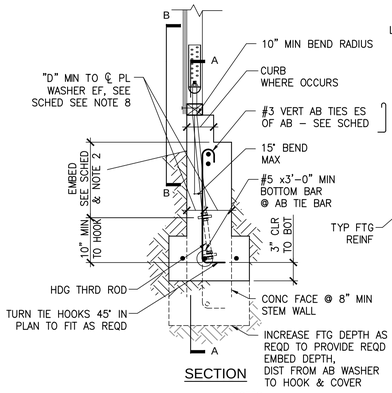
00BP-WD1-16



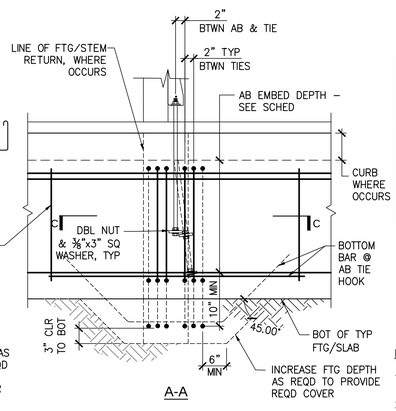
C-C PLAN



B-B



SECTION



A-A

TYPICAL HOLDOWN SCHEDULE (HDU)							
HOLDOWN	HOLDOWN ANCHOR HDG ROD SIZE	ANCHOR BOLT EMBED DEPTH (2)	SIDE DIST TO ℄ PL WASHER "D" (8)	VERT TIES @ AB		MIN POST WIDTH SIZE (4)	PULL TEST VALUE
				ES	TOTAL		
HDU2	⅝"	16"	4"	2	4	4x	7688#
HDU4	⅝"	16"	4"	2	4	4x	11413#
HDU5	⅝"	16"	4"	2	4	4x	14113#
HDU8	⅞"	16"	4"	3	4	6x	14950#
HDU11	1"	24"	6"	4	8	6x	23837#
HDU14	1"	24"	6"	4	8	8x	37313#

NOTES:

1. SEE PLAN & SHEARWALL SCHEDULE FOR LOCATIONS OF HOLDOWNS.
2. BOLTS SHALL BE EMBEDDED BELOW CURBS – DO NOT COUNT CURB HEIGHT FOR EMBEDMENT.
3. SEE GEN NOTES FOR SPECIAL INSPECTION REQUIREMENTS.
4. ALL HOLDOWN POSTS SHALL BE DF #1. USE SAME WIDTH POST AS WALL WIDTH. ALL HOLDOWN POSTS TO BE FULL HEIGHT. PROVIDE SHEAR WALL EDGE NAILING (AS NOTED IN THE SHEAR WALL SCHEDULE) TO ALL STUDS & POSTS WHICH HAVE HOLDOWNS AT TOP OR BOTTOM.
5. INSTALL ALL HARDWARE & ANCHOR BOLTS PER MANUFACTURER'S WRITTEN SPECIFICATIONS.
6. ANCHOR RODS SHALL BE THREADED HDG A307. SEE FDN ANCHOR BOLT DETAIL ABOVE. DO NOT USE SIMPSON (OR SIM) "SSTB" BOLTS.
7. FILL ALL HOLES W/ 3" MIN SDS SCREWS.
8. THICKEN STEM WALLS AS NECESSARY TO ACHIEVE SCHED. SIDE DIST TO ℄ PLATE WASHER. NOTIFY ARCHITECT & STRUCTURAL ENGINEER PRIOR TO PLACING REINF/FORMING WHERE CONFLICTS EXIST.

12 TYP FOUNDATION ANCHOR BOLT

SCALE: NTS

00BP-WD1-12

MARK	PLYWOOD STRUCT I	NAILING		FNDN SILLS (11)		BLKG ABOVE (A35) OR (LTP4)	SOLE PLATES		STUDS @ ABUTTING PANELS	REMARKS
		SIZE (1)	SPACING @ PANEL EDGE (2)	SIZE	BOLTING	SIMPSON ANCHOR	SIZE	REQD (4) ATTACHMENT		
10-6 V5	SEE PLAN FOR THK	10d	6" OC	3x	⅝"ø @ 32" OC	A35 @ 16" OC	2x	SDS ¼"x6" @ 12" OC	2x STUDS	
10-4 V5	SEE PLAN FOR THK	10d	4"	3x	⅝"ø @ 24" OC	A35 @ 12" OC	2x	SDS ¼"x6" @ 8" OC	3x STUDS	NOTE 5
10-3 V5	SEE PLAN FOR THK	10d	3"	3x	⅝"ø @ 16" OC	A35 @ 8" OC	2x	SDS ¼"x6" @ 6" OC	3x STUDS	NOTE 5
10-2 V5	SEE PLAN FOR THK	10d	2"	3x	⅝"ø @ 12" OC	A35 @ 6" OC	3x	SDS ¼"x6" @ 4" OC	4x STUDS	NOTES 5 & 12

NOTES:

1. COMMON NAILS ONLY–NO SINKERS ALLOWED. 10d (SHANK DIA=0.148"x 3" LONG) NAILS SHALL HAVE 1⅝" MIN PENETRATION.
2. NAIL SPACING @ INTERMEDIATE STUDS IS 12" OC, PROVIDE SOLID BLKG TO MATCH REQD STUD THICKNESS @ ALL UNSUPPORTED EDGES OF SHEATHING.
3. ALL STUDS SPACED @ 16" OC MAX. PROVIDE 3x OR 4x STUDS WHERE NOTED.
4. PREDRILL @ 90% x NAIL DIAMETER AS REQD TO PREVENT WOOD SPLITTING.
5. WHERE PLY IS APPLIED ON BOTH FACES OF WALL STUDS & NAIL SPACING IS LESS THAN 6" OC ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT STUDS & NAILS SHALL BE STAGGERED.
6. PLYWOOD TO BE EXPOSURE 1 (MIN) & CD SURFACED. SEE PLANS FOR FURTHUR REQUIREMENTS.
7. BOLT SPACING IS MAXIMUM. (2) AB's MIN @ EA SW–SEE TYPICAL SILL PLATE BOLTING DETAIL.
8. WHERE AT (E) WALLS ONLY: DOUBLE 2x STUDS MAY BE USED @ ABUTTING PANEL EDGES, UNO. WHERE DOUBLE 2x STUDS ARE REQUIRED @ ABUTTING PANELS, FACE NAIL STUDS TOGETHER w/ 16d, STAGGERED. SPACE @ SAME SPACING AS PLYWD PANEL EDGE NAILING.
9. PLYWOOD FOR SHEARWALLS SHALL BE 5–PLY & SHALL MEET APA STD PS–1.
10. ALL FASTENERS (NAILS, BOLTS, WASHERS, A35'S, LTP'S, ETC) IN CONTACT W/ PTDF TO BE HOT–DIP GALVANIZED.
11. STAGGER EDGE NAILING WHEN SPACING LESS THAN 3".

4 TYP SHEAR WALL SCHEDULE

SCALE: NTS

00BP-WD1-04

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

REVISIONS DATE

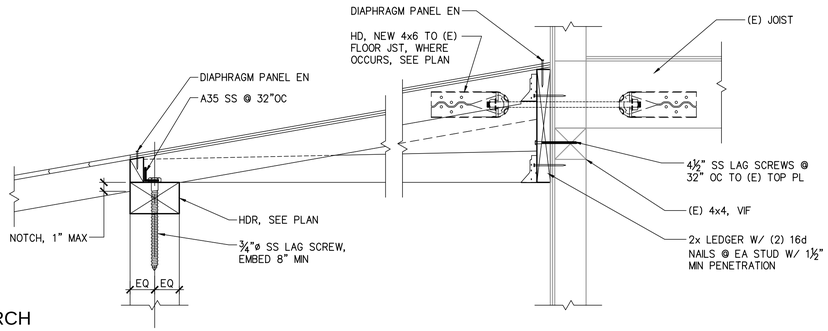
NAVARRO INN
NAVARRO RIVER REDWOODS STATE PARK
MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L. ROOD
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

REVISED 100% CD SET

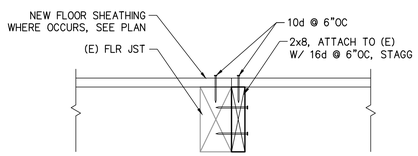
TYPICAL FRAMING DETAILS II

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S4.2
OF	10 SHEETS



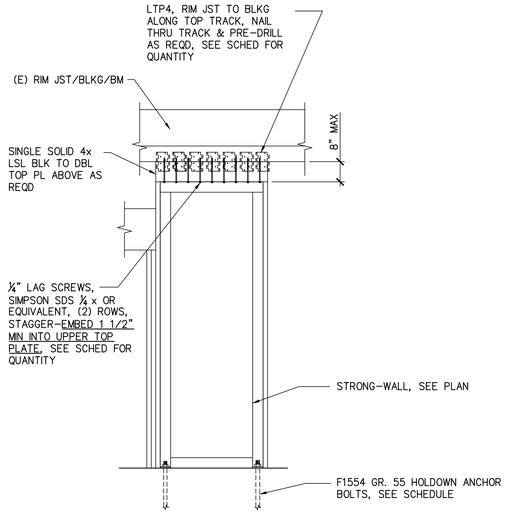
13 FRAMING @ PORCH

SCALE: 1 1/2" = 1'-0"



14 CONNECTION @ NEW FLOOR SHEATHING

SCALE: 1 1/2"= 1'-0"



NOTES:

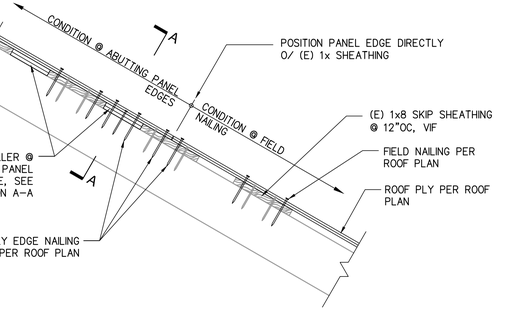
1. SEE PLANS FOR LOCATIONS & CONDITIONS
2. SEE STRONG–WALL SCHEDULE FOR ADDITIONAL NOTES & INFORMATION
3. SEE STRONG–WALL DETAILS FOR CONDITIONS ABOVE & BELOW FRAMES AS REQUIRED & CALLED OUT ON PLANS

11 TYP SIMPSON STEEL STRONG-WALL ELEVATION

SCALE: 3/4"= 1'-0"

6 GUSSET PL @ DIAGONAL STRAP BRACED FRAME

SCALE: 3" = 1'-0"



16 NEW PLYWOOD SHEATHING @ ROOF

SCALE: 1 1/2"= 1'-0"

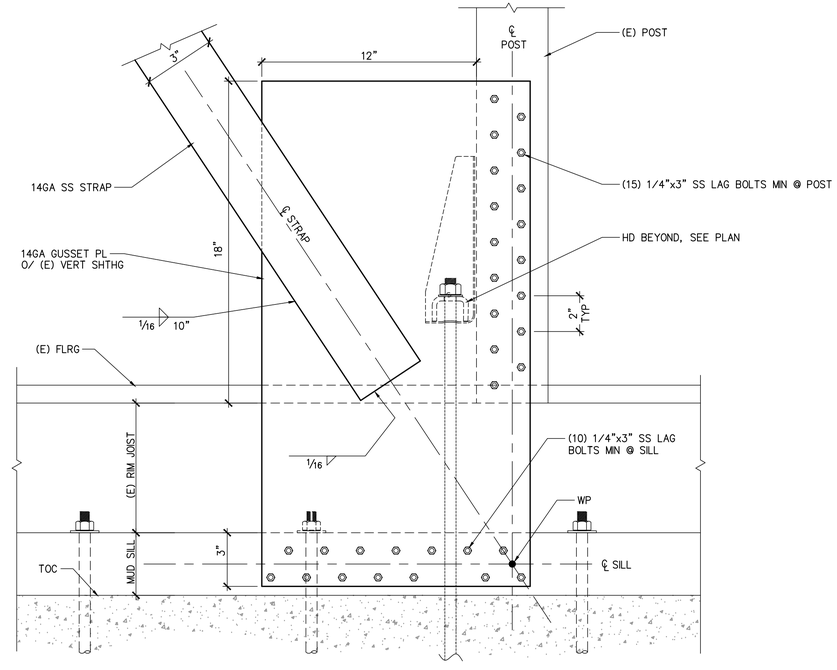
SECTION A-A

STRONG-WALL SCHEDULE					
FRAME TYPE	HD BOLT DIA	HD BOLT EMBED DEPTH	QTY SCREWS @ BOTTOM	QTY WOOD SCREWS @ TOP	QTY LTP4
SSW24x9	1"	24" (3)	N/A	14	6

1. THE MANUFACTURER SHALL SUPPLY INSTALLATION HARDWARE AND DETAILS WITH ALL FRAMES. STRONG–WALLS SHALL BE HANDLED, INSTALLED AND FASTENED PER MANUFACTURER'S INSTRUCTIONS. WHERE MFR'S INSTRUCTIONS DIFFER WITH THE DRAWINGS, CONTRACTOR SHALL CONTACT THE ENGINEER FOR RESOLUTION
2. ANCHOR BOLTS SHALL BE TYPE F1554 GR. 55 AND SHALL BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS USING THE TEMPLATES PROVIDED

12 STRONG-WALL SCHEDULE

SCALE: 3/4"= 1'-0"



8 GUSSET PL @ DIAGONAL STRAP BRACED FRAME

SCALE: 3" = 1'-0"

FULCRUM
STRUCTURAL ENGINEERING
665 THIRD ST. STE 333
SAN FRANCISCO, CA 94107
TEL: 415.543.0161
FAX: 415.543.0162

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MENDOCINO COUNTY, CA

REGISTERED PROFESSIONAL ENGINEER
ALEX L. ROOF
No. S 004492
Exp. 12/31/12
STRUCTURAL
STATE OF CALIFORNIA

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LATERAL FRAMING DETAILS

DATE	May 16, 2011
SCALE	AS NOTED
DRAWN	FSE
JOB	08-37
SHEET	S5.1
OF	10 SHEETS